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# **Incorporating Advanced Signal Control Systems into an Archived Data User Service Program**

*by*

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16. Abstract <p>The deployment of Intelligent Transportation Systems, which incorporate some type of data collection or traffic surveillance capabilities, has been rapidly increasing over the past decade. More recently it has been recognized that these data collection systems can be used as additional sources of mobility data, augmenting traditional data sources such as relatively sparsely distributed permanent count stations and supplementary 48 hour volume counts. Most of the research conducted to date has focused on data archiving systems that have freeway system data collection equipment as their primary data source. However, little is known about the feasibility of using advanced signal control systems (ASCSs) as sources of mobility data. A possible cause for the relative inattention to the use of advanced signal system equipment is that using an intersection as a source for road segment volume counts is contrary to conventional traffic data collection principles, which stipulate that road segment volume counts should be taken outside the influence area of intersections.</p> <p>The purpose of this research was to determine if data collected from an advanced arterial signal control system can be used to generate information that is useful for transportation engineering analyses other than signal optimization and control. This research also discusses some of the technical challenges and limitations to using data collected by an arterial signal control system and presents an analysis of the validity of the data. The premise investigated is that data from signal control system surveillance equipment can be used to calculate daily volume counts for a roadway segment if the surveillance equipment is deployed in an acceptable configuration. This premise is tested by screening and aggregating data from signal system data collection equipment and comparing it with data from traffic monitoring equipment located in close proximity to the intersections being analyzed.</p> <p>The results of this research support the conclusion that reasonable volume estimates can be generated from system detectors located upstream of the intersection stop bars, if the system detectors are deployed on all major approach through lanes. This research also demonstrated that the utility of ITS data requires more than simply the deployment of ASCS or other ITS data collection equipment. In order for an ITS archived data management system to be successfully implemented, the data product needs of the end users must be considered in the design and deployment of the traffic monitoring and control system, as well as the data management system.</p>			
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# **FINAL REPORT**

## ***INCORPORATING ADVANCED SIGNAL CONTROL SYSTEMS INTO AN ARCHIVED DATA USER SERVICE PROGRAM***

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## ABSTRACT

The deployment of Intelligent Transportation Systems which incorporate some type of data collection or traffic surveillance capabilities has been rapidly increasing over the past decade. More recently it has been recognized that these data collection systems can be used as additional sources of mobility data, augmenting traditional data sources such as relatively sparsely distributed permanent count stations and supplementary 48 hour volume counts. Most of the research conducted to date has focused on data archiving systems that have freeway system data collection equipment as their primary data source. However, little is known about the feasibility of using advanced signal control systems (ASCSs) as sources of mobility data. A possible cause for the relative inattention to the use of advanced signal system equipment is that using an intersection as a source for road segment volume counts is contrary to conventional traffic data collection principles, which stipulate that road segment volume counts should be taken outside the influence area of intersections.

The purpose of this research was to determine if data collected from an advanced arterial signal control system can be used to generate information that is useful for transportation engineering analyses other than signal optimization and control. This research also discusses some of the technical challenges and limitations to using data collected by an arterial signal control system and presents an analysis of the validity of the data. The premise investigated is that data from signal control system surveillance equipment can be used to calculate daily volume counts for a roadway segment if the surveillance equipment is deployed in an acceptable configuration. This premise is tested by screening and aggregating data from signal system data collection equipment and comparing it with data from traffic monitoring equipment located in close proximity to the intersections being analyzed.

The results of this research support the conclusion that reasonable volume estimates can be generated from system detectors located upstream of the intersection stop bars, if the system detectors are deployed on all major approach through lanes. This research also demonstrated that the utility of ITS data requires more than simply the deployment of ASCS or other ITS data collection equipment. In order for an ITS archived data management system to be successfully implemented, the data product needs of the end users must be considered in the design and deployment of the traffic monitoring and control system, as well as the data management system.

## ***INTRODUCTION***

Over the past two decades, a large number of Advanced Traffic Management Systems (ATMS) and Advanced Traveler Information Systems (ATIS) have been developed and deployed throughout the United States and abroad. An estimated \$209 billion will be invested in Intelligent Transportation Systems (ITS) programs between now and the year 2011 (ITSA, 2001). The ATMS and ATIS systems have been deployed at the local, statewide, and multi-state corridor levels and have been in use long enough to demonstrate their effectiveness at managing traffic and providing travelers with information about traffic conditions (Mitretek, 2000). ATMS and ATIS systems typically use a variety of equipment such as cameras, automatic vehicle detection systems and inductive loop detectors to collect the data used for system monitoring, incident management, and traveler information. This information is usually relayed to a Traffic Management Center (TMC) where transportation agency personnel monitor and manage the transportation system.

In general, TMC staffs are focused on their core mission of transportation system efficiency, and lack the resources and expertise to manage the data they collect such that it can be accessed and used effectively for purposes other than real time operations (Winick, 2000). Often TMCs either do not archive data, or the data is stored only for a short period of time. Surveys by Texas Transportation Institute researchers indicate that 20 percent of TMCs around the country were not archiving data at all, and there was no consistency or data archiving standards among the others (Turner, 1997).

There are ongoing efforts to address this issue, and to this end the U.S. Department of Transportation Joint Program Office for Intelligent Transportation Systems (JPO) has developed a strategic plan to broaden the use of archived data collected by ITS equipment. The objectives of this plan can be summarized as follows (USDOT, 2001): to integrate ITS derived data with other data sources, make it more accessible to end users, and perhaps most importantly, to transform the raw data into information useful to practitioners in many areas of the transportation profession.

Most of the efforts to date towards the goal of integrating ITS data into the broader traffic data environment have been focused on data collected by freeway management systems. However, advanced signal control systems that use traffic surveillance equipment are being deployed with increasing frequency. As with the freeway ATMSs, the data from signal control systems can be archived and used for purposes other than signal control, however the integration of signal control systems and data management systems is rarely implemented. The need for more research on coordination between signal control systems and archive data management systems was addressed in the *ITS Benefits: Data Needs Update 2000* report published by the JPO. The *Data Needs Update* report presents the areas where additional information about the benefits of ITS services is desired and was derived from an ITS America workshop on data needs. Of the 19 research needs identified by the data needs task force, Information Management – Data Archiving was ranked second, with incident management being only a slightly higher priority (Mitretek, 2001).

ITS deployment studies conducted by the JPO indicate that a significant percentage of transportation agencies have deployed advanced signal control systems with data collection capabilities, and the use of such technology is projected to increase (USDOT, 2001). Typically these systems collect data such as traffic volumes and speeds using inductive loop detectors. In cases where the data is stored for a period of time, seldom is the data screened for validity or aggregated and transformed into information such as daily traffic counts, Annual Average Daily Traffic (AADT) or other measures. This would be advantageous in that departments of transportation (DOTs) traditionally deploy and maintain a separate infrastructure, usually referred to as traffic monitoring systems, to collect AADT and other basic measures. Despite the relative lack of attention to date, the increasing deployment of advanced signal control systems suggests that a significant potential exists for advanced signal control systems to be used as a source of traffic information. This research assesses this potential through a review of published literature concerning the use of ITS data as a resource for transportation engineering information, and a case study of the advanced signal control system deployed by the Virginia Department of Transportation (VDOT) in the Northern Virginia region.

## ***PURPOSE***

The purpose of this research is to determine if data collected from an advanced arterial signal control system can be used to generate information that is useful for transportation engineering analyses other than signal optimization and control. The premise is that data from signal control system surveillance equipment can be used to calculate daily volume counts for a roadway segment if the surveillance equipment is deployed in an appropriate configuration. This premise is tested by screening and aggregating data from signal system data collection equipment and comparing it with data from traditional traffic monitoring equipment located in close proximity to the intersections being analyzed.

Thus, the specific goal of this research is to determine if traffic surveillance equipment deployed in a signal control system can be used to estimate daily traffic volume counts. This research discusses some of the technical challenges and limitations of using data collected by an arterial signal control system and presents an analysis of the validity of the data. The exploration of these issues involving intelligent signal systems as a data resource will help to address one of the top research needs identified in *ITS Benefits: Data Needs Update* report published by the JPO.

## ***SCOPE***

The signal control system used in this research effort's analysis is deployed in the VDOT Northern Virginia district, which includes the counties of Arlington, Fairfax, Loudon, and Prince William and referred to as the Northern Virginia Smart Traffic Signal System (NVSTSS). Arlington County, unlike the other counties in the Northern Virginia District, is responsible for their street system. All data collected by "system" detectors of the NVSTSS (system detectors are installed upstream of typical intersection queues to collect basic system demand and



performance data) are stored in an archival database at the University of Virginia and Virginia Transportation Research Council's Smart Travel Laboratory (STL). The data analyzed in the NVSTSS archival database were detector volume, speed, and occupancy, and the daily volume counts from the intersections were compared to daily volume counts collected from traffic monitoring equipment used by VDOT's Traffic Monitoring System (TMS). The TMS uses a network of permanently installed surveillance equipment (permanent count stations) and regular temporary counts collected with portable equipment to produce estimates of traffic volumes on all roads maintained by VDOT.

## METHODOLOGY

The following methodology guided the research effort.

1. Literature review. *A review of the literature was conducted to provide a foundation for the research. The literature review focused on traditional traffic data collection programs, the ITS archived data user service, and advanced data management practices.*
2. NVSTSS Data Preparation. *This task involved preparing the system detector data in the NVSTSS archival database for comparison with daily volume counts collected by the TMS permanent count stations. The design (usually referred to as the "schema") of the NVSTSS archival database was initially intended only to support long-term storage of the data, and, as such, extracting useful information from it required complex, SQL queries that often took hours to run. The schema was altered by screening the system detector data for validity and screening the sites to determine which ones had system detectors in an appropriate configuration for daily volume analysis, and then storing the results of this data screening in additional fields and tables. Tables were also created to store aggregate information, in preparation for Task 3, below. A more detailed description of the key aspects of this task are presented below.*

### 1.1.1 Detector Data Validity

The data files that the STL receives from the NVSTSS contain 15 minute sums of volume, 15 minute averages of speed, and 15 minute averages of loop occupancy for each loop detector in the STSS. These data files can contain invalid records due to equipment or communications failures, which are sometimes manifested by values of negative 1. More commonly however, the invalid records are not so easily identified, and a series of screening tests must be applied to test the validity of the data.

The data screening tests used in this task are based on either threshold values, traffic flow theory, or a combination thereof, and are applied with SQL commands to the database. The formulae for the screening tests are as follows (Hauser, 2001):

**Prescreening tests:**

1. Volume AND Occupancy AND Speed  $\geq 0$
2. Volume  $< 3100$  AND Occupancy  $< 100$
3. Volume  $\geq$  Occupancy

**Feasible Volumes:**

- |                                      |                                   |
|--------------------------------------|-----------------------------------|
| 4. IF Occupancy = 0 OR 1             | THEN Volume $< 580$               |
| 5. IF $1 < \text{Occupancy} \leq 15$ | THEN $1 < \text{Volume} < 1400$   |
| 6. IF $15 < \text{Occupancy} < 25$   | THEN $180 < \text{Volume} < 2000$ |
| 7. IF Occupancy $\geq 25$            | THEN Volume $> 500$               |

The first test screens out records that have negative values; typically negative values indicate some kind of equipment or communications error. The second test ensures that volumes and occupancies will not exceed maximum thresholds of 3100 vehicles per hour and 100%, respectively. Tests four through seven screen out records for which the combination of volume and occupancy is inconsistent with traffic flow theory relationships of volume and density. Essentially these tests say that the volumes recorded must be consistent with the density measured by the occupancy recording.

### **1.1.2 Detector Configuration**

The second step in data preparation task was to identify the sites that were appropriate for analysis. There are nearly 1000 intersections in the NVSTSS, and the STL archival database receives data from 438 of them. Currently, only data from system detectors is transferred to and archived by the STL, which is why the STL does not receive data from all of the intersections controlled by STSS. Among the sites that have system detectors, the number and location of the system detectors installed varies considerably. Some sites have system detectors at only some of the through lanes on one or two of the intersection approaches. Other intersections have system detectors on all the approaches, and a few even have system detectors in left turn lanes.

In order for an intersection to be useful for traffic volume studies described in this research, the system detectors must be installed on every through lane on the route or street for which the volume count is desired. For example, if there are 4 through lanes on the major approach, there must be four system detectors installed on the major street approaches in order for all major approach through traffic to be counted. If there are system detectors on the minor approaches, the traffic on the minor street approaches can also be counted. Figure 1 illustrates an intersection with system detectors on all approaches. It is rarely the case that all major and minor approach through lanes of an intersection have system detectors installed. Figure 2 illustrates a more common configuration, where only the major street approaches

have system detectors. The percentage of intersections with all main street approaches instrumented is still relatively low, and most intersections have some combination of major and minor lanes with system detectors, as illustrated in Figure 3. In order to simplify the queries used in Task 3, data aggregation, a new table called "good sites" was created which contains the site identification numbers of all the intersections that have a system detector installed on each major approach through movement.

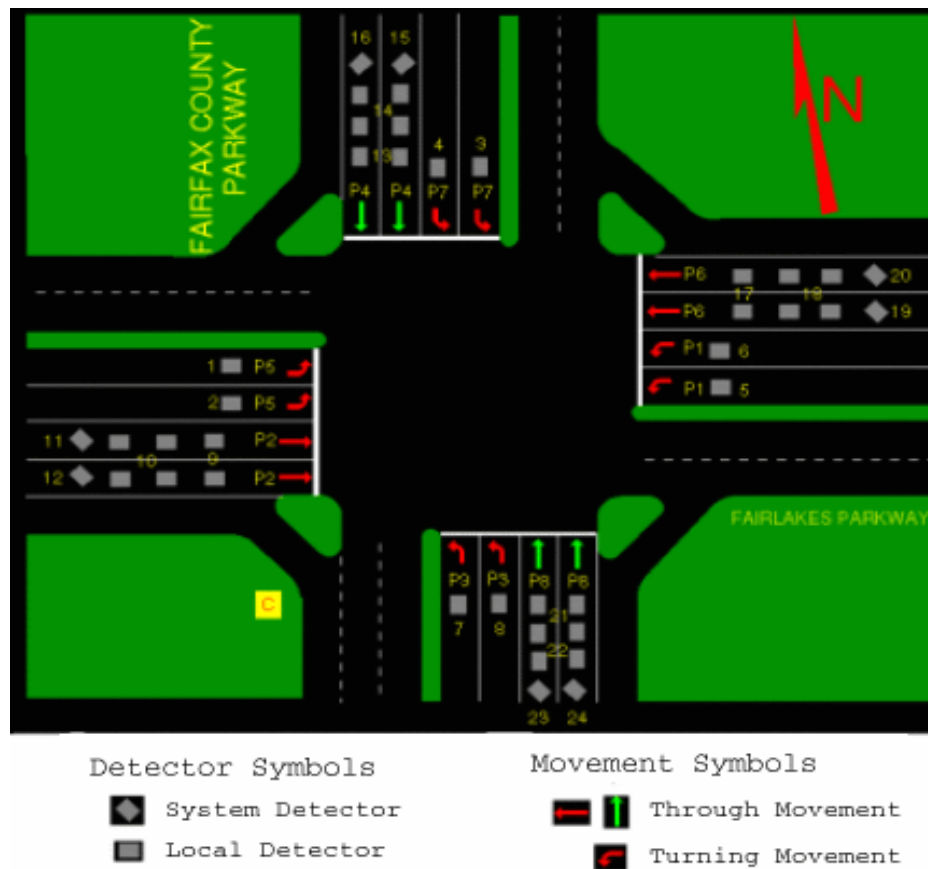


Figure 1: An intersection with system detectors on all approach through lanes.

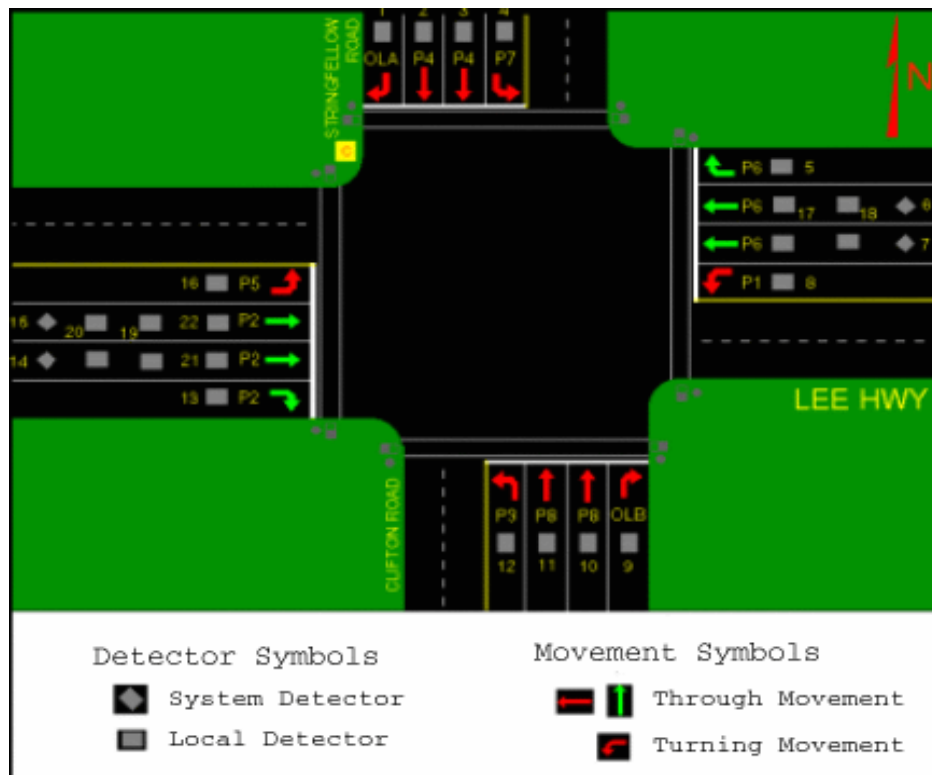


Figure 2: An intersection with system detectors only on the major approach through lanes.

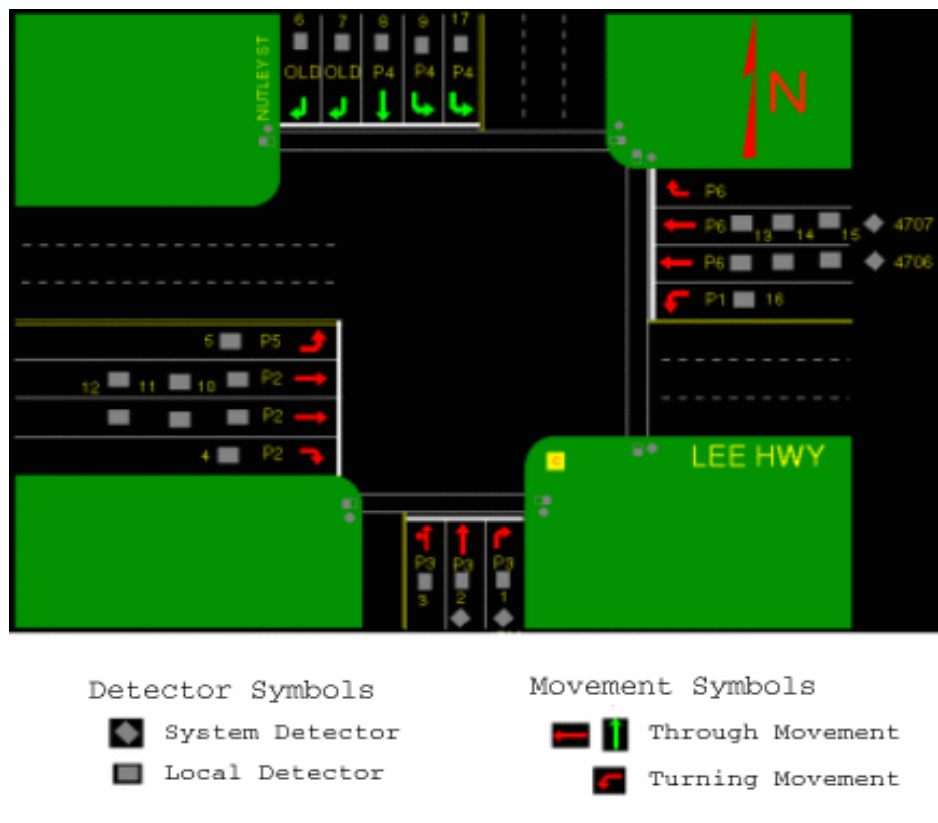


Figure 3: A typical system detector configuration

3. *Data Aggregation.* Daily volume counts are the basis for AADT counts, and many transportation performance measures. This task produced daily volume counts using archived data from the NVSTSS. In order for daily major approach volume counts at an intersection to be accurate, all of the collection intervals for all of the major approach system detectors must contain valid records. Since the collection interval is 15 minutes, every major approach through movement system detector must have 96 valid records in the “detector\_data” table. If some of the intervals contain invalid data, or the detector does not report any data, the volume count will be erroneously low. The aggregate table “site\_daily\_vol,” which contains daily volume counts for the major approaches of the intersections in the table “good\_sites,” was created with the following procedure:

- The major approach through movement system detectors at the intersection were identified
- For major approach through movement detector, for each day, the number of valid records was determined
- If all detectors reported 96 valid records for a day, the volumes for each time period for each detector were summed.
- The total volume, the day it was recorded, and the siteid was added to the “site\_daily\_vol” table.

4. *Acquisition of Baseline Data.* After the system detector data was aggregated into daily volume counts for each intersection, a baseline data source was needed with which to compare the signal system daily volume counts. VDOT's network of permanent count stations, the TMS described in a previous section, was the ideal choice for this level of analysis because data were already collected and archived. Having the archived data available meant that approximately 400 daily volume counts could be compared on a day-for-day basis between the TMS and NVSTSS. Furthermore, because data from the TMS is used to calculate the officially published traffic volumes on VDOT highways, the TMS data was considered to be the “correct” volume against which to compare the daily volumes calculated from the signal system data.

5. *Location Selection and Evaluation.* The final step in preparing the data for comparison of daily volumes was to select locations from both data sets where valid comparisons could be made. In addition, the road and intersection geometry of these locations were examined in more detail.

### 1.1.3 Site Identification

The locations of all the NVSTSS controlled intersections and the TMS permanent count stations were obtained from VDOT staff. The data was analyzed with Geographic Information System (GIS) software to identify the permanent count stations that are located less than one mile from, and on the same route as an intersection that has an appropriate system detector configuration.

## 1.2

### 1.2.1 Geometric Evaluation

The threshold distance of one mile was used based on an inspection of the data available, and with the expectation that as the distance between the intersection and permanent count station increased, the difference in daily volumes observed would also increase. Another factor that can contribute to differences in daily volume between an intersection and its corresponding permanent count station is the configuration of the system detectors with respect to the intersection geometry. The intersection diagrams from MIST and aerial photography were used to assess how well the data from the system detectors should represent the total volume on the roadway segment.

6. Daily Volume Comparison. *The final step in this research was to compare the daily volumes calculated from the NVSTSS system detectors with the daily volumes recorded by the TMS permanent count stations. The TMS and NVSTSS volume counts constitute two observations of the same random variable, the traffic volume on a roadway segment. As stated previously, the TMS data, which is the source of officially published volume counts, was used as the base-line, thus the percent error,  $D_n$ , between the two data sets is as follows:*

$$D_n = \frac{(X_n - Y_n)}{X_n}$$

where:  $X_n$  = TMS volume for day  $n$   
 $Y_n$  = signal volume for day  $n$

A 99 percent confidence interval was then calculated for the mean of each set of  $D_n$  values. The formula for the confidence interval,  $CI$ , for the mean difference between observations variable as follows:

$$CI = d \pm \frac{(t_{\alpha/2, n-1})(s_d)}{\sqrt{n}}$$

where:

$d$  = mean percent error

$t_{\alpha/2, n-1}$  =  $t$  distribution statistic for confidence level of  $1-\alpha$

$s_d$  = standard deviation of percent error

$n$  = number of samples



The true mean error between observations will lie within the limits of the confidence interval computed from the formula above. In other words, one can be 99% confident that the mean difference in daily volumes between the intersection and permanent count station will be within the limits of the confidence interval.

## 2 RESULTS

### 3 Literature Review

The literature review provided a foundation for the project. Findings from the literature review are summarized below.

#### *3.1 Traffic Data Collection Programs*

Traditional TMSs have been focused on the collection of data to support non real-time analyses of transportation systems. Examples of such analyses include pavement performance monitoring, safety analyses, evaluations of structure and bridge sufficiency, and long term highway system performance monitoring for use in transportation planning, land use and economic decision making and some aspects of environmental quality analyses. These analyses are focused more on annual growth trends over many years, or average seasonal, or day of week variations, rather than real time measures of mobility. The data elements collected have traditionally been traffic volume counts, speeds, vehicle classification counts, and the weight of various types of vehicles on the highway. The vehicle weights and vehicle classification counts are very important in pavement performance monitoring and the classification counts are needed for air quality, noise and emissions studies.

The TMS programs usually rely on inductive loop detectors embedded in the pavement. The speed, vehicle classification, and weight data collection requires additional equipment such as additional loops, axle sensors, and or piezos. Various combinations of these devices that can be installed to produce the requisite data are discussed in the *AASHTO Guidelines for Traffic Data Collection Programs*. (AASHTO, 1992). In addition to using different equipment configurations, TMS's typically have deployed permanent data collection stations, often at intervals of tens of miles, and used portable data collection devices such as pneumatic tubes to collect data on road segments between the permanent count stations. The permanent count stations collect and report data continuously. The portable data collection equipment is typically used to collect data over a 48 hour or similar interval, and then the data is retrieved and the data collection equipment is installed at another location. A given location may be counted in this manner every two or three years, depending on the functional classification of the roadway. The data obtained from the portable equipment is transformed from 48 hour volume counts to AADT estimates by applying seasonal and other adjustment factors which are calculated from data collected by the permanent count stations (Chapparral Corp. 2001).

The *AASHTO Guidelines for Traffic Data Programs* (AASHTO, 1992) provide guidance on the entire traffic data collection process, including assessing data collection needs, equipment deployment, and analysis activities such as editing, summarizing, reporting and retaining data. The *Guidelines* specify for example, how and where permanent and portable data collection equipment should be deployed: on straight segments of roadway, away from the influence of intersections and entrances that might effect the stable flow of traffic on the segment of road under observation. The *Guidelines* also define the procedures for aggregating raw traffic data

into useful statistics and annual estimates such as Annual Average Daily Traffic (AADT). These procedures, attempt to minimize the effect of missing or erroneous data, which according to the *Guidelines*, should never be imputed if the data are to be used to provide information products to the end users of an archived data management system.

#### 3.1.1.1 Archived Data User Service – National ITS Architecture

### 3.1.2

The National ITS Architecture is an ITS program development framework intended to guide the implementation of ITS. The National Architecture is subdivided to the following principle components (USDOT, 2001):

- *User Services*, which represent what the system will do from the perspective of the user. A user might be the public or a system operator. In version 3.0 of the National Architecture, there are 31 user services, which are organized into seven user service groups, called “bundles” : Travel and Traffic Management, Public Transportation Management, Electronic Payment, Commercial Vehicle Operations, Emergency Management, Advanced Vehicle Safety Systems, and Information Management.
- The *Logical Architecture*, which is best described as a tool that assists in organizing complex entities and relationships. It focuses on the functional processes and information flows of a system. Developing a logical architecture helps identify the system functions and information flows, and guides development of functional requirements for new systems and improvements.
- The *Physical Architecture*, which is the physical (versus functional) view of a system. The physical architecture provides agencies with a physical representation (though not a detailed design) of how the system should provide the functionality defined by the user services. It is divided into subsystems which represent the major physical components of ITS infrastructure, such as roadside data collection equipment, in vehicle sensors and traffic monitoring centers.

The user service that directly addresses the needs of traffic data collection programs is the Archived Data User Service (ADUS). Other user services, such as Advanced Traffic Monitoring Systems (ATMS) and Advanced Signal Control Systems (ASCS) typically collect vast amounts of data. The potential uses of this data, beyond real time operation and system monitoring prompted the inclusion of ADUS into the National ITS architecture in order to address information management. ADUS specifies the functions that an ITS archived data management system should provide. The ITS architecture identifies 16 user groups of the ADUS in the broad functional categories of planning, operations, safety, and research.

Typically the functionality specified by the ADUS is provided by the implementation of an archived transportation data management system (ADMS). Version 3.0 of the National ITS Architecture includes Data Management as a subsystem of the Physical Architecture, and describes its function as follows (USDOT, 2001):

*The Archived Data Management Subsystem collects, archives, manages, and distributes data generated from ITS sources for use in transportation administration, policy evaluation, safety, planning, performance monitoring, program assessment, operations, and research applications. The data received is formatted, tagged with attributes that define the data source, conditions under which it was collected, data transformations, and other information (i.e. meta data) necessary to interpret the data. The subsystem can fuse ITS generated data with data from non-ITS sources and other archives to generate information products utilizing data from multiple functional areas, modes, and jurisdictions. The subsystem prepares data products that can serve as inputs to Federal, State, and local data reporting systems. This subsystem may be implemented in many different ways. It may reside within an operational center and provide focused access to a particular agency's data archives. Alternatively, it may operate as a distinct center that collects data from multiple agencies and sources and provides a general data warehouse service for a region.*

## **3.2**

### **3.3 Advanced Data Management**

Simply collecting traffic data does not fully support transportation analyses. The raw data must be transformed into information that is “integral to the transportation practice” (USDOT, 2001), and this information must be readily accessible to end users. In order to meet these goals, the data archives that store the raw data and derive information from it must be carefully designed. In order to empower end users to access information when and how they most need it, traditional data archives, i.e. databases that are designed for capture and storage of data, need to be redesigned or modified. The simplest way to do this is to screen, aggregate, and otherwise transform the data into information needed by users. Once this transformation is accomplished, the new information could be stored in additional tables. The advantage to storing the information in new tables is that the screening and aggregation procedures, which can be quite complex and time consuming, need only be performed once, and users can then access the information directly, without having to perform calculations on the raw data. User empowerment could be further enhanced by developing predefined queries and reports, or ultimately developing a Graphical User Interface (GUI) to the data tables which allow users to access information without having to learn database programming or structured query language (SQL) (Corey, 1998).

A more robust means of delivering vital information derived from raw data to users is to migrate the database to a data warehouse. A data warehouse is a information management system that is specifically designed for decision support, as opposed to data capture or transaction processing (Corey, 1998). Data in a data warehouse may be organized or segregated into data marts, which are specialized subsets of data that support decision makers in a particular user group.

Data warehouses often depart from traditional database designs in that user defined information needs take precedence over minimizing data storage space and data redundancy. Data warehouses schemas tend to have fewer tables and relationships, and typically would only store the raw data and the descriptive data used to aggregate the raw data. To perform aggregation and other analyses, data warehouses make use of On Line Analytical Processing (OLAP) tools which help end users quickly and easily access the information they need. In

effect, the OLAP tools eliminate the need to aggregate the raw data and store it in new tables, as described above (Corey, 1998).

## **4 NVSTSS Data Preparation**

The design (or schema) of the NVSTSS archival database was modified to support this research. The schema had to be modified because it was initially designed for the primary purpose of data capture and storage. The large amounts of disaggregate data in the database made it difficult to generate aggregate measures such as daily volume counts, let alone traffic statistics and performance measures based on daily volume counts. The revised schema presented is perhaps the most expedient way to facilitate the production of information useful to stakeholders because it only involves creating new tables to store information and running SQL queries once or at scheduled intervals to populate them. The revised NVSTSS schema is shown in Figure 5.

While the database architecture described here can provide end users with information they need, the process of acquiring ITS data, and distributing information derived from it could be further improved by implementing a formal data warehouse. In this type of data management system, “staging areas” for receiving and processing raw data from the TMCs would be separated from the data marts of mobility information. On Line Analytical Processing (OLAP) tools would enable users to quickly and easily analyze the raw data and derive any important information they needed, potentially through a web browser interface.

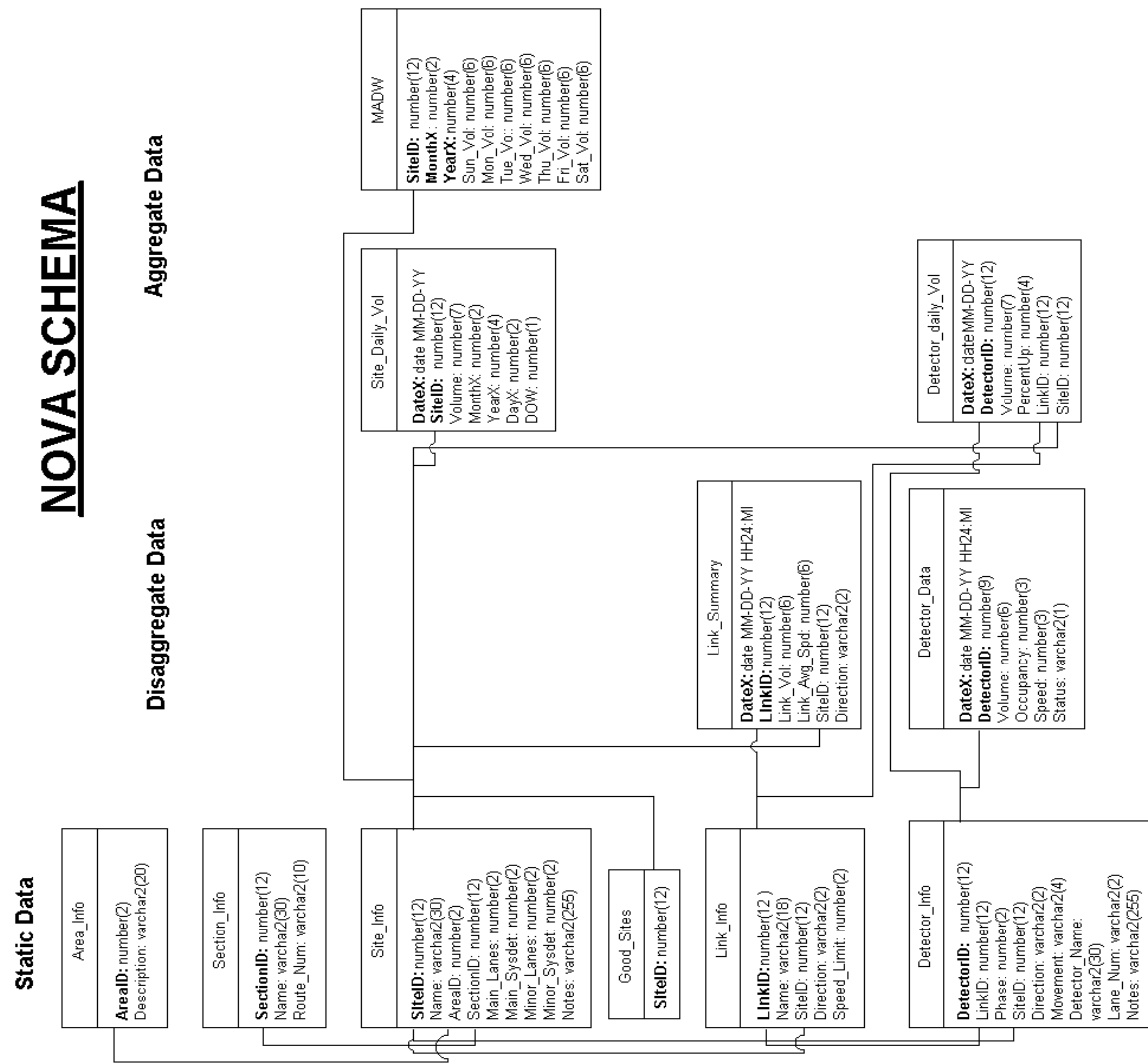


Figure 4: Current NOVA Schema.

Rather than running the series of screening tests every time a user needs valid data from the database, a “status” field was added to the data table, “detector\_data.” This field has a value of “1” for every fifteen minute record which passes the screening tests described in the methodology. Fields describing the lane and system detector configuration at an intersection were added to the table “site\_info” to facilitate identification of intersections from which valid daily volume counts can be calculated. The fields Main\_Lanes and Minor\_Lanes indicate the number of through lanes on the main and minor approaches of the intersection, respectively. The fields Main\_Sysdet and Minor\_Sysdet indicate the number of main and minor approach through movement system detectors at the intersection, respectively. Using these new fields, the table “good\_sites” was created, which lists the intersections that have a system detector on each major approach through lane.

Other tables were added to store aggregate data and information calculated from the detector data. The tables listed below were added to the schema to facilitate data analysis and distribution. Discussions with VDOT stakeholders indicated that many users are interested in a few common aggregate measures such as AADT and average volumes for a given day of week in any given month. It is more computationally efficient to create tables to store this aggregate data, than to have users issue the queries which perform the aggregation each time they are interested in a traffic statistic such as the AADT at a location.

- **Detector\_Daily\_Vol:** This table contains a daily sum of the detector volumes for the detectors listed in table Good Sites. PercentUP is a decimal value of how many valid records are recorded by the detector each day. A valid record is one which has Detector\_Data.Status = 1.
- **Link\_Summary:** This table contains link volume and average speed data, one record per link every 15 minutes. Link\_Vol is volume in vehicles. Link\_Avg\_Spd is in mph. This data is calculated for sites in the table Good\_Sites, from the data in the table Detector\_Data where Detector\_Data.Status = 1.
- **MADW:** This table contains the Monthly Average Day of Week volumes. The data are calculated for the sites listed in the table Good\_Sites. Sun\_Vol - Sat\_Vol are equal to the average value of the sum of the volumes in vehicles, for the respective day in a given month. Assuming at least one valid day (Sunday - Saturday) for each month is available, there are 84 records per site per year. A valid day is one where all main line general purpose system detectors report valid values for the entire day (Detector\_Data.Status = 1)
- **Site\_Daily\_Vol:** This table contains a daily sum of the main line volumes for the sites listed in the table Good\_Sites, from the data in the table Detector\_Data where Detector\_Data.Status = 1. Monthx, Yearx, Dayx, and DOW are numeric values for month, year and day, and day of week and are included to eliminate the need to perform time consuming data type conversions when calculating temporal statistics.

## **Detector Data Validity**

The main data table of the archived NVSTSS database, “detector\_info” is populated with 15 minute aggregates of system detector volume, speed and loop occupancy. The database contains data for 216 discontinuous calendar days, from a total of 428 intersections in Northern Virginia. Of the 26,340,524 records in the “detector\_info” table, 1,565,533, or six percent, fail the screening tests described in the previous chapter. These invalid records are retained in the database for archival purposes and potential future use, but are not used in the analyses presented in this report.

#### 4.1.1.1 Detector Configuration

In order for a daily volume count to be calculated from the intersection system detectors, a system detector must be installed on each through lane of the route for which the volume count is desired, only 162 of the 428 intersections from which the STL receives data have system detectors on each major approach through lane.

## 5 Data Aggregation

Daily volume data was aggregated to the “site\_daily\_vol” table. This table contains data from 162 intersections, and 122 discontinuous calendar days. The “site\_daily\_vol” table has so few intersections and days compared to the “detector\_data” table because of the screening procedures described above. Only intersections which have system detectors on all major street through lanes were included, and furthermore, if any of the detectors at that intersection reported at least one bad record, the volume for that day was not added to the table.

### *5.1 Acquisition of Base-Line Data*

There are less than fifteen VDOT maintained arterial permanent count stations in the Northern Virginia region. GIS analyses reveal that six of these are within one mile of and on the same route as an intersection where the arrangement of system detectors is such that daily volumes can be estimated. The valid daily volume counts at each of these locations during the year 2000 were downloaded from VDOT's TMS database. These daily volume counts were loaded into a database that also contained daily volume counts from the STL database.

### *5.2*

### *5.3 Location Selection and Evaluation*

As mentioned above, there are six permanent count stations located within one mile of and on the same route as a NVSTSS intersection with the proper system detector configuration. One of these six permanent count stations is equidistant from two intersections, which yields a total of seven possible study locations.



All of the intersections analyzed are located in Fairfax County. Routes 1, and 50 are US Highways, while Routes 7, 28, and 7100 are part of Virginia's primary road system. The “Distance” and “Number of Access Points” columns in Table 1, below, were calculated using GIS maps obtained from VDOT's Northern Virginia District GIS staff. “Number of Access Points” indicates the number of access points between the intersection studied and the corresponding permanent count station.

Major Approach Route #	Intersection Name	Distance (mi)	Number Access Points
28	Sully Rd.Eleanor Lawrence Park entrance	0.27	2
7100	Fairfax Co. Pkwy & John Kingman Rd.	0.18	0
1	Richmond Hwy. & Woodlawn Blvd.	0.36	1
7	Leesburg Pike & a commercial entrance	0.65	10
7100	Fairfax Co. Pkwy & Lee Jackson Hwy.	0.36	2
50	Lee Jackson Hwy. & Stringfellow Rd.	0.3	0
7100	Fairfax Co. Pkwy & Fair Lakes Blvd.	0.4	0

Table 1: List of intersections which meet the selection criteria.

***5.3.1 One location pair, the intersection of Lee - Jackson Memorial Hwy. and Stringfellow Rd could not be analyzed because all the volumes recorded in the TMS database were invalid on the the days that the NVSTSS database contained records. The intersection of the Fairfax County Parkway and Fair Lakes Parkway could not be used in the analysis either, because there was only one day of valid data in the NVSTSS database for that location.***

It is to be expected that the differences in daily volume counts between the intersection and its corresponding permanent count station will increase with the distance and number of access points between them. Errors due to the presence of traffic sources and sinks between the intersection and count station could be positive or negative, depending on whether the net change in volume results in an increase or decrease in the traffic volume at the two locations. Therefore, differences in volume due to traffic sources and sinks are not really errors, but rather represent actual differences in traffic volume at the two locations. The configuration of the intersection system detectors is another principle source of error. At some of the intersections studied, the system detector layout is such that large numbers of vehicles will not be counted, since they enter the roadway segment from turning lanes on a minor approach. At these locations, the error should be positive, indicating that the volume calculated from the intersection system detectors is less than the volume counted by the nearby permanent count station. With these concerns in mind, each of the five locations used in this research are evaluated below:

### 5.3.2 Site 1 - Route 28: Sully Rd & Eleanor Lawrence Park

Figures 6 and 7 show that this intersection has only one minor approach, the entrance to a local park. The minor approach volumes are likely to be quite low compared to the major approach volumes; it is therefore unlikely that the configuration of the system detectors will contribute significantly to any error observed. Errors recorded at this location are expected to be the result of actual differences in traffic volumes between this intersection and the count station.

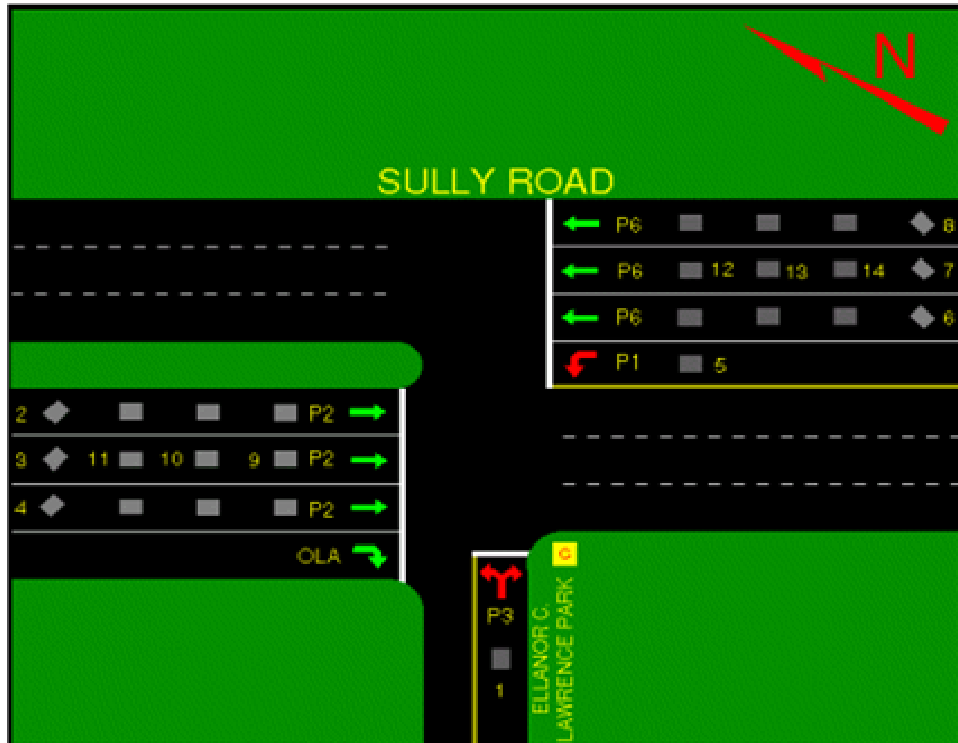


Figure 5: intersection diagram for Sully Rd & Eleanor Lawrence Park



Figure 6: Aerial photograph of Sully Rd and Eleanor Lawrence Park entrance.

5.3.3 Site 2 - Route 7100: Fairfax County Pkwy & John Kingman Rd.

At this intersection, inspection of the geometry shown in the Figures 8 and 9 reveals that traffic on a total of five major approach lanes, including one double left turn lane, will not be counted by the system detectors. There are no entrances between this intersection and the corresponding count station, and the distance is the least of all the locations sampled (0.18 mi). It is therefore likely that the difference in volumes at this location is due to the intersection geometry and the arrangement of system detectors. Thus errors are expected to be positive, meaning an undercount due to vehicles entering the roadway segment from lanes without system detectors.

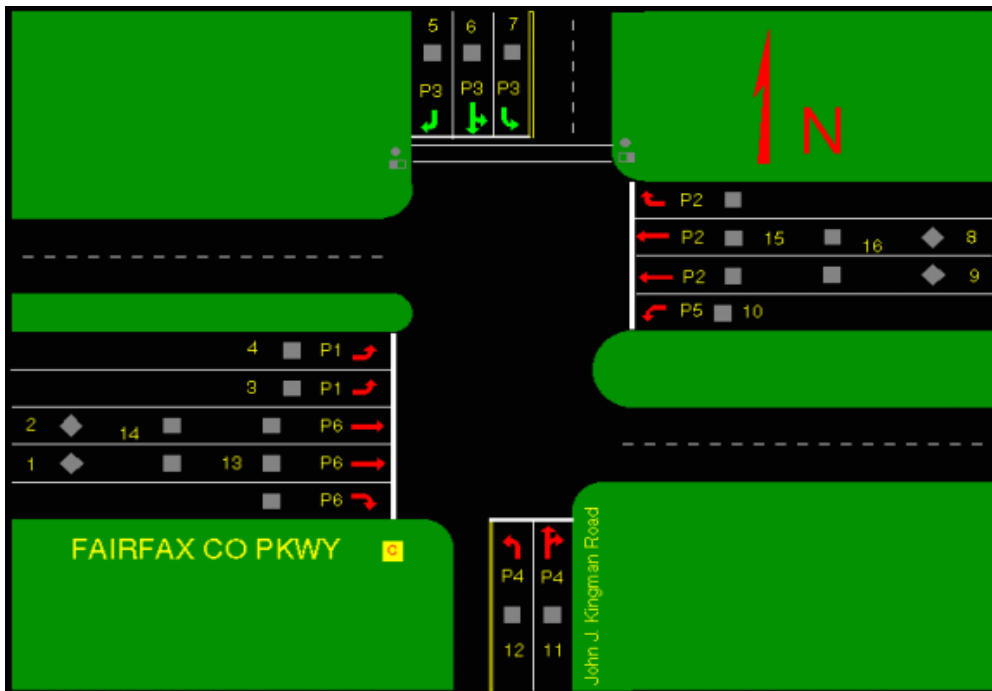


Figure 7: Intersection diagram of Fairfax Co. Pkwy & John Kingman Rd.



Figure 8: Aerial photograph of Fairfax Co. Pkwy & John Kingman Rd.

This intersection, like the intersection of Sully Road and Eleanor Lawrence Park has only one minor approach, which leads to a cultural recreation area, in this case Woodlawn Plantation. It is therefore unlikely that intersection geometry, shown in Figures 10 and 11, will contribute significantly to the error, and errors recorded will more likely be the result of actual differences in traffic volume between the two locations.



Figure 9: Intersection diagram of Richmond Hwy. & Woodlawn Blvd.





Figure 10: Aerial photograph of Richmond Hwy & Woodlawn Blvd.

5.3.5 *Site 4 - Route 7: Leesburg Pike and Tyson's Corner Entrance*

The aerial photograph, Figure 13 clearly shows that this intersection is at a major commercial entrance, (Tyson's Corner Shopping Center), and the intersection diagram, Figure 12, shows five approach lanes whose traffic would be missed by the system detectors. Errors at this site could be due to the interchange and the 0.65 miles between the intersection and the permanent count station and due to the system detector configuration. Errors at this study location are expected to be higher than at other locations studied in this report.

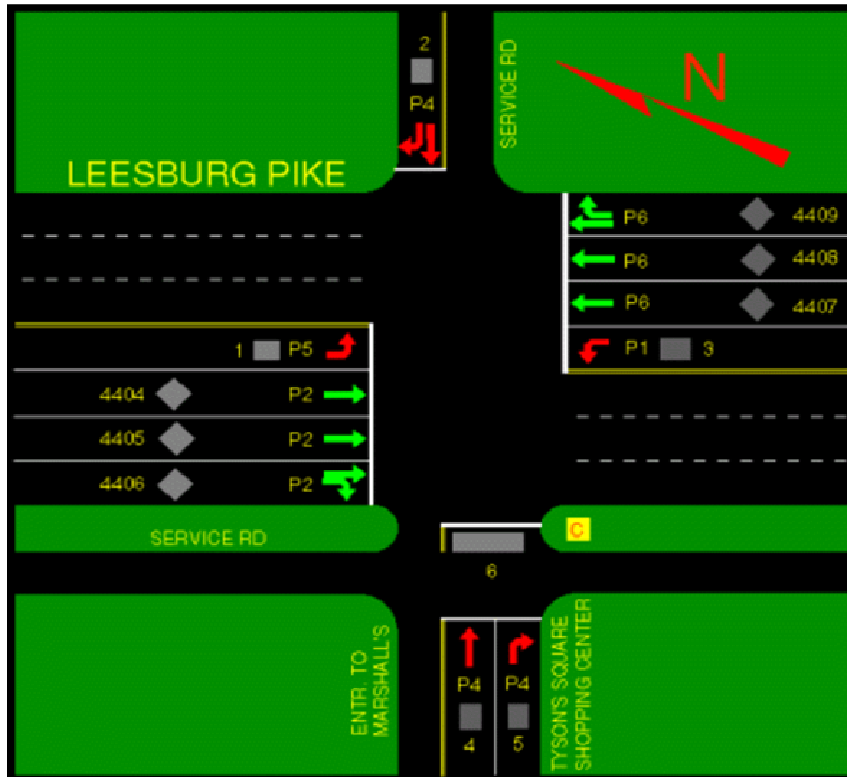


Figure 11: Intersection diagram for Leesburg Pike & a commercial entrance.



Figure 12: Aerial photograph of Leesburg Pike & a commercial entrance.

5.3.6 Site 5 - Route 7100: Fairfax County Pkwy & Lee – Jackson Hwy.

Inspection of the aerial photograph, Figure 15 shows that this intersection is part of a grade separated interchange. The intersection diagram, Figure 14 shows that none of the traffic on the ramps is counted by the system detectors. These ramps lead to Lee - Jackson Highway, a major arterial. It is therefore likely that the error is due to the system detector configuration and not the distance or number of entrances between the intersection and the corresponding permanent count station.

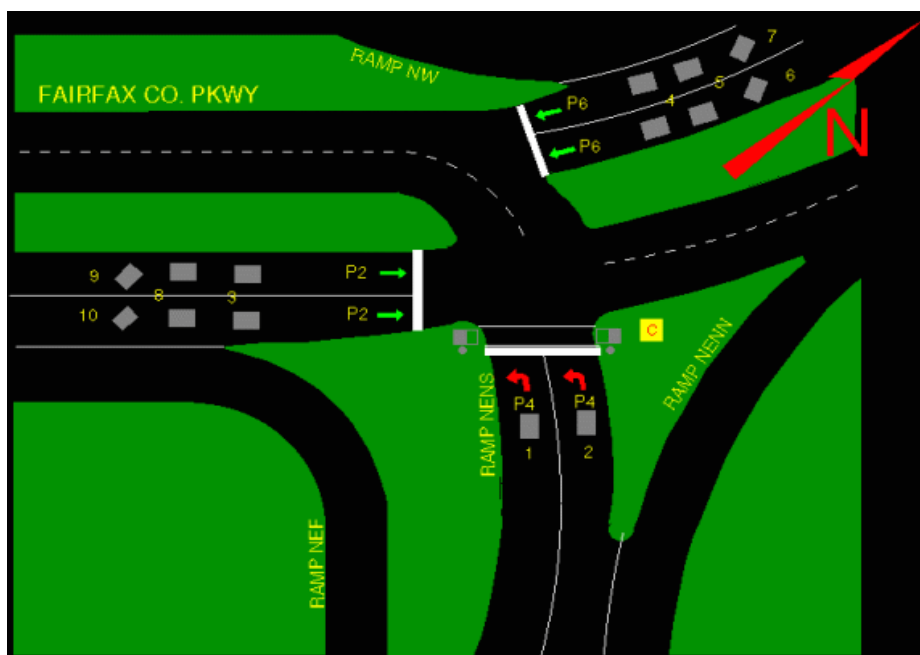


Figure 13: Intersection diagram for Fairfax Co. Pkwy & Lee Jackson Hwy.





Figure 14: Aerial photograph of Fairfax Co. Pkwy & Lee - Jackson Mem. Hwy.

**5.4**

**5.5**

### ***5.6 Daily Volume Comparisons***

Table 2 presents the average daily volumes at each of the five sites from the NVSTSS and TMS, the mean percent error, population standard deviation of the percent error and the 99% confidence interval for the mean errors. Errors which are negative indicate that the NVSTSS volume counts are higher than the TMS volume counts, while positive errors indicate that the signal system count is lower than the adjacent TMS volume count. There is a 99% probability that the true mean error will be between the lower and upper limits of the confidence interval presented in the table. The “Sample Size” column indicates the number of daily volume counts analyzed. The sample size is smaller than might be expected because the analysis presented in this research uses only data that have passed the screening tests described in the methodology, and because computer network problems limited the data available for study to approximately six months of volume counts.

Site	Average Volumes		Percent Error		# of Days	99% Confid. Interval	
	TMS	NVSTSS	Mean	Std Dev		Lower	Upper
1	51675	54124	-4.71	0.93	8	-5.85	-3.56
2	13351	12516	5.84	2.51	93	5.16	6.53
3	31606	34457	-9.07	2.07	106	-9.59	-8.54
4	69564	79311	-13.92	1.63	33	-14.69	-13.14
5	46450	40215	13.98	3.90	35	12.18	15.77

Table 2: Summary of results

Aside from equipment errors and other problems which result in inaccurate data in the databases, there are two main variables which contribute to the difference in volumes collected by the pairs of permanent count stations and intersection counters listed in the previous section. One cause of error is traffic sources and sinks between the permanent count station and the intersection. Major access points on the highway segment between each permanent count station and intersection pair were counted using GIS maps. The distance between the intersection and count station can also be used as a proxy measure for traffic sources and sinks. The advantage of using distance, rather than counting entrances on a map, is that the map may not represent all access points such as driveways and commercial entrances. The second contributing factor to the differences in traffic volumes is the layout of the intersection system detectors and the intersection geometry. The relationships between the number of access points, distance, and intersection geometry are discussed in more detail below.

#### 5.6.1.1 Access Points

The most obvious explanation for differences in daily volume counts at two locations along a route is that between the two observation points vehicles have entered or exited the route segment under study. The number of access points between the intersection and their corresponding count stations were counted from GIS maps. The relationship between the number of identified access points and the absolute value of the mean error is presented in Figure 16.

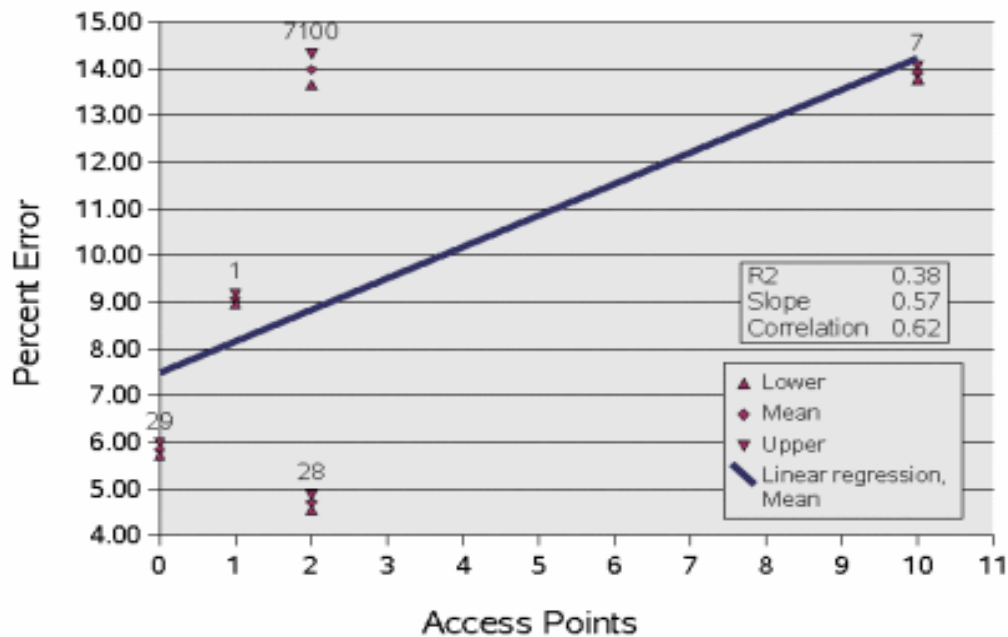


Figure 15: Number of access points vs percent absolute error.

The graph shows that there is a very little correlation between the number of access points and the percent error in the volume counts. In fact, there are two locations, on Routes 28 and 7100 which have the same number of access points (2), yet have a difference in percent error of over 60%.

#### 5.6.1.2 Distance

Since available maps may not show all private and commercial entrances, the relationship between percent error in volumes and distance from the count stations to the intersections was also analyzed. Distance thus becomes a surrogate measure for the traffic sources and sinks between the intersection and the count station. Figure 17 shows the relationship between distance and percent error.

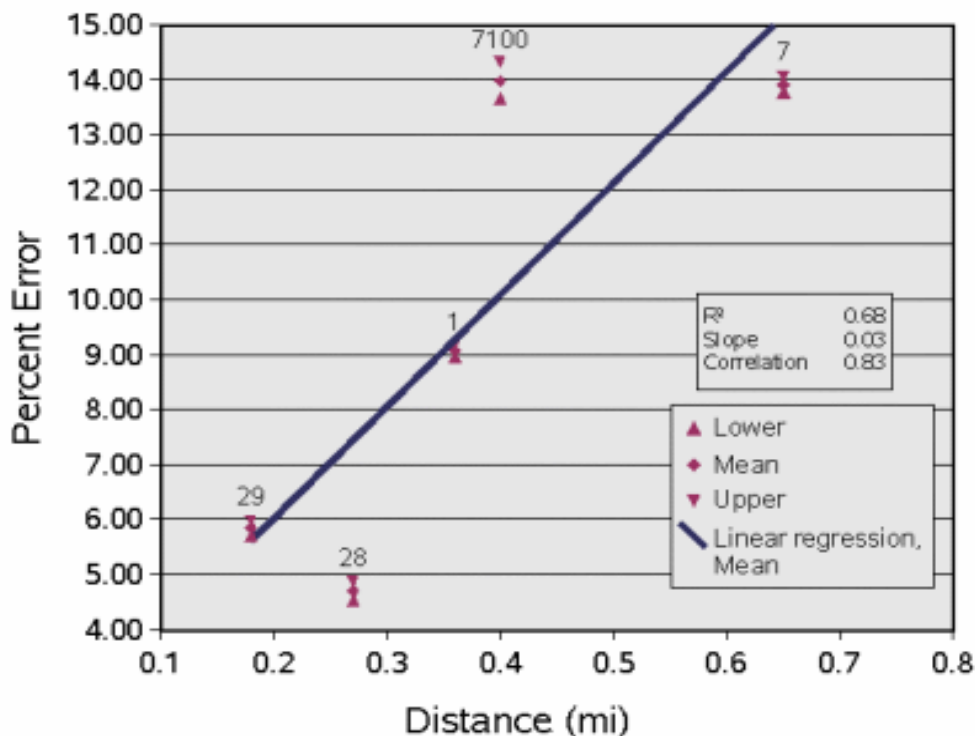


Figure 16: Distance vs absolute percent error.

There is a better correlation between distance and percent error than there is between the number of entrances and the percent error, and the fit of the linear regression line to the mean data points is better. As with the relationship between access points and distance, there are two data points with nearly same distance value (0.4 miles), yet with high differences in error (30%).

#### 5.6.1.3 Intersection Geometry

In addition to traffic source and sinks between the intersection and the permanent count station, the configuration of the system detectors at the intersection can result in inaccurate counts. Because only major approach system detector data was available, the intersection itself represents a potential traffic source or sink because the systems detectors are arranged upstream of the intersections. Vehicles turning onto the major street from the minor streets enter the segment downstream of the detectors, thus they are not counted. Since system detectors are located downstream of the taper point of turning lanes, vehicles which are turning off the major approach are also not counted. Errors introduced by system detector configuration will thus always be positive, which means an under count. The potential for vehicles not being counted is best analyzed by examining the intersection diagrams and aerial photographs. These images were presented and discussed in the preceding section.

### 5.6.2

### 5.6.3

### 5.6.4 *Comparison Summary*

Of the five study locations analyzed, the volume counts from two intersections were lower than the volume counts recorded at the nearby permanent count stations. This is to be expected based on the intersection geometry and system detector configuration. An analysis of the arrangement of the system detectors and the intersection geometry at the intersection of Fairfax County Parkway and John Kingman Road and the intersection of Fairfax County Parkway and Lee – Jackson Memorial Highway shows that at these sites, the difference in volume counts is probably caused by vehicles entering and exiting the roadway segment from lanes which do not have a system detector.

At the other three locations, Sully Rd and Eleanor Lawrence Park, Richmond Highway and Woodlawn Boulevard, and Leesburg Pike and the entrance to Tyson's Shopping Center, the volumes reported by the signal system were higher than the volumes reported by the continuous count station. The resulting negative errors are most likely to be caused by vehicles leaving the traffic roadway segment at points located between the intersection and the permanent count stations, since errors due to system detector configuration will always be positive. The relationship between the percent error and distance for these three locations is shown in Figure 18 below. Note that once the intersections where system detector location is likely to be the primary source of error are removed, the strength of the correlation between distance and error improves dramatically.

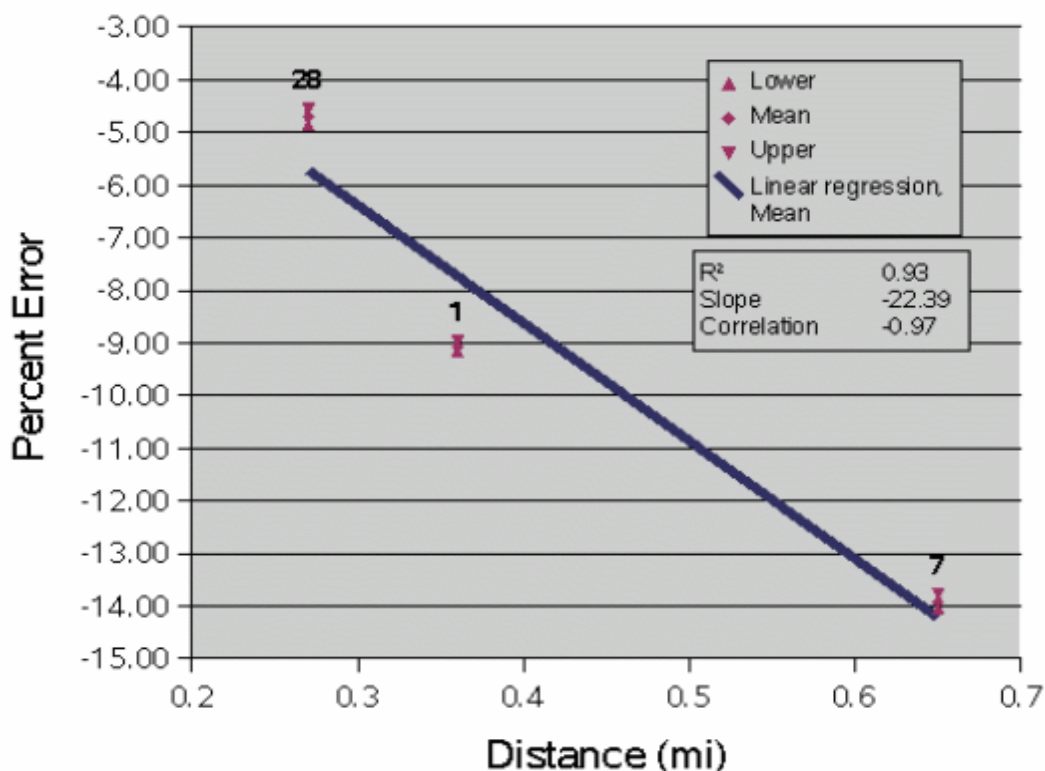


Figure 17: Distance vs percent error, for intersections where detector configuration is not likely to be a significant source of error.

## 6 CONCLUSIONS

Based on the results of this research, the following conclusions are offered.

1. The National ITS Architecture specifies the functionality of the ADUS and its associated elements in great detail. However, as the ADUS is a relatively new user service, there is little information available from the FHWA or other sources about how this functionality can be achieved with existing and future deployments of travel and traffic management systems. A great deal of emphasis has been placed on overcoming the institutional barriers to ADUS implementation by conducting workshops, focus groups, and interviews with stakeholders. However, in order for the ADUS functionality specified by the National Architecture and envisioned in the JPO strategic plan to be fully manifested, more practical guidance is needed on how to resolve the technical barriers such as surveillance equipment deployment and configuration, data management system design, and user interface deployment.

2. This research demonstrates that ADUS functionality does not inherently result from successful advanced signal system deployment. Although advanced signal systems frequently have data collection capabilities, the usefulness of the data depends heavily upon the configuration and deployment of the data collection equipment. Furthermore, this research shows that ADUS functionality requires more extensive deployment of surveillance equipment than is currently the practice in advanced traffic control systems.
3. In addition to the equipment considerations, the data as collected by the signal control system needs a great deal of processing to translate it into information such as transportation system performance measures. This processing entails screening the data for validity and aggregating the data spatially and or temporally, as specified by the data product needs of the end users.
4. Transactional databases, while necessary for efficient capture and initial storage of detector data, cannot adequately provide the functionality required by the ADUS. As mentioned above, a significant amount of data processing is required to generate the archived data products such as daily traffic counts, VMT, and other performance measures. To fully achieve the ADUS functionality specified in the National ITS Architecture, a complete archived data management system would have to be deployed. Such a system should have a transactional database to acquire the data from the field equipment, a "staging area" database where the raw data is screened and prepared for inclusion into the final component, the data warehouse. The data warehouse component provides both data analysis functions, and serves as the foundation for the user interface.
5. Upstream signal system detectors can be used to generate accurate daily volume counts, However, as in the case of VDOT's NVSTSS, reliance on upstream system detectors may severely limit the number of intersections that can be used to generate daily volume counts. This is because not all intersections have upstream detectors on every approach through lane. Furthermore, the accuracy of the volumes calculated from upstream system detectors depends upon the configuration of the system detectors with respect to the overall intersection geometry. In cases where the traffic entering or exiting the roadway link from the approaches without system detectors represents a significant portion of the total volume, the volume calculated from the upstream system detectors will be erroneously low. This can be particularly problematic when the signal is part of an interchange, because the ramps typically do not have system detectors installed on them.
6. Daily volume counts calculated from signal system data collection equipment may be difficult to validate without actually conducting a traffic count immediately adjacent to the intersection under study. This research demonstrates that validating intersection daily volume counts by comparing them to daily volume counts from nearby permanent count stations can be complicated by the fact that the actual traffic volumes at the two locations could be different, depending on how many vehicles enter or exit the roadway between the intersection and the permanent count station.

The distance, and the number of access points identified on maps between the intersection and permanent count station may be used as predictors of the magnitude of difference in traffic volume.

7. If the NVSTSS intersections controlled by the MIST system can be successfully integrated into an ADMS, a substantial savings in data collection costs may be realized by VDOT. Currently there are only fifteen arterial permanent count stations in the Northern Virginia District, thus most of the traffic volumes published by VDOT are based on 48-hour traffic volume counts. These counts entail the installation and removal of temporary equipment, which is hazardous and costly. By using the data from the NVSTSS intersections, VDOT may be able to reduce the number of 48 hour counts needed, thus reducing the costs of the traffic monitoring program.

## RECOMMENDATIONS

**The following specific recommendations are presented to VDOT and FHWA.**

1. The needs of ADUS stakeholders should be considered when designing the data collection components of an advanced signal control system. Specifically, the need to deploy additional detectors, beyond what is required for signal control and optimization, should be carefully investigated, and the location of detectors with respect to intersection geometry should also be examined. Ideally, when a new system is being deployed, the design for the detector installation should be such that all traffic volumes on the roadway links can be accurately counted and used in an archived data management system. This may entail the addition of comparatively few detectors, or merely an adjustment to their location with respect to intersection geometry. For existing systems, the feasibility of using additional existing detectors, such as local detectors, should be compared with the cost and utility of installing additional system detectors.
2. A complete data management system, with subsystems for initial data collection and storage, data screening and validation, and data aggregation and analysis needs to be developed for each major VDOT traffic control system in order to "maximize the integration of information with other data sources and systems" (USDOT, 2001). For example, an ADMS should have a database for initial data capture and storage, a separate "staging area" database for screening and validating the data, and a data warehouse which stores the validated data. The OLAP and graphical user interface tools of the data warehouse should ensure that the system meets the principles of accessibility and pertinence by providing end users with a graphical tool to analyze the data according to their specific needs.
3. Additional data should be acquired from the NVSTSS, either by deploying additional system detectors, or by using local detectors. The objective of this additional data acquisition would be to produce a better volume count by capturing vehicles that are currently not being counted. If local detectors are determined to be sufficiently



reliable and feasible to acquire, their use could dramatically expand the number of intersections where daily volume counts can be calculated. In addition, local detector data would enable turning movement counts, and volume counts for minor approaches to be calculated.

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